PLANNING AND SCHEDULING OF CONSTRUCTION IN COMERCIAL BUILDING BY USING PRIMAVERA (P6)

M. kamalnaath , Dr. T.Felix kala

M.Tech construction Engg and management dept of civil engg Dr.M.G.R.Educational &research Institute ,INDIA PROFESAOR  department of civil engg Dr.M.G.R.Educational &research INSTITUTE

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Why construction planning and scheduling are important

Proper construction planning and scheduling are important in ensuring that your construction project gets completed on time and within budget. A thoroughly planned construction schedule not only outlines the pace of your work but it dictates how your work gets done. It also helps define your processes, methods, and sequences for when materials are put in place.

Preparing your construction schedule meticulously and ahead of time maximises your efficiency and productivity. As your construction schedule allows you to improve your quality control measures, it is effortless to sequence work and to ensure you have the correct quality and quantity of materials used in each step.

Materials and resources procurement is on track as you can use your schedule to purchase the right materials exactly when you need them. Safety performance is improved as you use your schedule to track which worker is on site and make sure that protection guidelines are properly followed.

Having a reliable construction schedule also allows you to allocate your time better among all your project stakeholders, which helps them plan their activities better. By getting total control of your project, you reduce unpleasant surprises, making it easy to avoid cost overruns and delays.

What is construction planning?

Construction planning is essential in managing and executing your construction projects as it involves selecting the technology, defining the work tasks, estimating the required resources and extent of individual tasks, and identifying possible interactions and workflows among different activities.
An efficient construction plan is fundamental in setting your budget and schedule for the entire work needed. Creating and developing the construction plan is a highly challenging and critical task in construction management.

You have to develop the technical aspects and on top of that, you have to make organisational decisions about relationships between project stakeholders and even the subcontractors you will have to include.

However, it isn’t enough just to track and use construction data for a single project. Real estate developers especially benefit from storing their data over a long period. For example, one of the best reasons to maintain CRE data (or commercial real estate data) is because it allows for better decision-making.

According to The Constructor, a civil engineering informational resource website, there are three major types of construction project planning:

1. Strategic planning

It involves a high-level selection of project objectives. Strategic planning is usually done by the project owner’s corporate planners. In order to achieve the owner’s project goals, they decide what project to build and the completion deadline with the project teams developing the master construction execution plan that falls within the guidelines set in the strategic and contracting plans.

2. Operational planning

It involves detailed planning by the construction teams to meet the project's strategic objectives. Before the project teams can detail the construction schedule, they have to go through a series of questions so they can prepare the construction master plan:

- Will the operational plan meet the strategic planning target date?
- Are sufficient construction resources and services available within the company to meet the project objectives?
- What is the impact of the new project on the existing workload?
- Where will we get the resources to handle any overload?
- What company policies may prevent the plan from meeting the target date?
- Are usually long delivery equipment or materials involved?
- Are the project concepts and design firmly established and ready to start the construction?
- Is the original contracting plan still valid?
- Will it be more economical to use a fast-track scheduling approach?

3. Scheduling

It involves a detailed operational plan set on a time frame as per the strategic objectives.

What are the first steps in planning a construction project?

The classic approach to developing a construction plan, which is the basis for modern construction planning, is based on the 1998 published book, Project Management for Construction by Chris Hendrickson. The common development strategy is to adopt a primary emphasis on either cost control or on schedule control.

Construction planning may be cost or expense oriented, or schedule oriented. With cost-oriented project planning, there is a distinction between costs incurred directly in the performance of an activity and indirectly for the accomplishment of the project. Indirect costs may include borrowed expenses for project financing and overhead items. For schedule oriented planning, the emphasis is on the schedule of project activities over time, and this is considered critical.

Read also: How to ensure your 3-6 week planning will be delivered on time

The planning is focused on ensuring that proper precedences among activities are followed and maintained and that scheduling of resources is done in an efficient manner. This results in critical path scheduling procedures (the maintenance of seamless workflows) and job shop scheduling processes (the efficient use of resources over time). Whichever your construction planning is centred on, effective delivery, schedule, and budget is always intertwined and are both major concerns.
Once you have figured out your planning emphasis, it is time to consider all other functional requirements for your construction planning.

1. **Choosing which technology and construction methods to utilise.** Your choice of the right technology and construction methods are critical aspects in the success of your project execution. Your decision whether to make concrete structures on site or order pre-fabricated ones will directly affect the cost and duration of tasks involved in the construction process. Finding the right digital solution for your project will be decisive for how productive your team is as it will directly affect the time it takes for the various activities to be completed and the flow from one activity to another by cutting down unnecessary administrative tasks.

2. **Defining work tasks and activities.** Because construction planning determines your construction scheduling, defining various work tasks is vital in framing the schedule of your construction activities. In that way, you can estimate the resources needed and timetable the required sequences and critical paths among tasks. Defining appropriate work tasks is tedious but a necessity in applying formal scheduling processes and in standardising specific tasks. Once tasks are defined correctly, a hierarchy of activities emerge which can be visualised like this example of activities in a roadway project plan:

3. **Defining relationships and critical flow among activities.** After work activities are defined, you can now specify the relationships among them. Precedence relations between tasks and activities mean that activities must happen in particular sequences. Numerous natural sequences exist for construction activities due to requirements for structural integrity, regulations, and other technical requirements.

4. **Estimating activity durations.** Remember, each work activity is associated with time duration and these durations are the bases for preparing the schedule. All formal scheduling relies on duration estimates as well as the defined precedence relations. A realistic estimation coupled with historical records of particular tasks and activities is critical in avoiding delays.

5. **Estimating resource requirements for work activities.** Besides precedence relations and time durations, resource requirements are also estimated for each activity. By correctly estimating resource requirements per activity based on their comprehensive definitions, particular resource requirements for the entirety of the project can be also defined while avoiding issues with resource allocation and procurement problems.

6. **Establishing a coding system.** Having a coding system for each of the identified activities allows for better integration of organisation efforts and better information flow. A coding system allows you to standardise definitions and categories of items and activities between projects and among project stakeholders. Coding systems also make it easy to retrieve historical data of cost, productivity, and duration of your activities. Couple this with a construction management software that keeps all your data in a central location makes your coding system even more efficient.

These steps are needed to develop a proper construction plan and allow you to transform your plan into a schedule. Construction planning is not limited to the period after you have been awarded a contract. It should be an essential and continuous activity even during your facility design.

**What are the five phases of construction?**

There are five major phases in a standard construction project where construction planning and scheduling play critical roles:

1. **Initiation.** In this phase, your project idea is evaluated to determine its feasibility and whether or not, it should be undertaken. This is the beginning of the life of a project where the project objectives are identified and defined.

2. **Planning.** This phase includes the further development of the project and outlining the details needed to meet the project goals. In this phase, you identify all the work needed to be done, the tasks and resources required, and the strategy to make all of them possible. Usually, a project budget is prepared by the project manager to provide cost estimates for labour, equipment, and materials. Once every planning detail is determined, they should all be documented in a quality plan that also shows the targets, assurance, control measures, building codes, and even customer criteria. By this time, the project would be ready for execution.

3. **Execution.** Welcome to the implementation phase where the project
plan is put into motion on site. Project control and communication are essential during the execution phase. The project manager has the power to control the project’s direction through progress reports and activity performance. Any deviation from the plan has to be addressed and corrected. The goal of action is to never deviate from the original plan that’s why progress should always be reported on time.

4. **Monitoring.** The monitoring phase happens simultaneously with the execution phase and covers all progress and performance measurement related to tracking and ensuring that the project is going according to the construction plan and schedule.

5. **Closure.** Once all project aspects have been delivered and the client has agreed, the project is now ready for closure. This phase involves all activities related to the final handover to the customer.

**Conclusion**

To be successful with your construction projects, careful planning is needed to create a schedule that will allow you to deliver your projects on time and within budget. Both construction planning and scheduling take a lot of time to create and implement but the time you will save during the actual work will be more than the time you spent creating your plan and schedule.

**Construction Planning**

**9.1 Basic Concepts in the Development of Construction Plans**

*Construction planning* is a fundamental and challenging activity in the management and execution of construction projects. It involves the choice of technology, the definition of work tasks, the estimation of the required resources and durations for individual tasks, and the identification of any interactions among the different work tasks. A good construction plan is the basis for developing the budget and the schedule for work. Developing the construction plan is a critical task in the management of construction, even if the plan is not written or otherwise formally recorded. In addition to these technical aspects of construction planning, it may also be necessary to make organizational decisions about the relationships between project participants and even which organizations to include in a project. For example, the extent to which sub-contractors will be used on a project is often determined during construction planning.

Forming a construction plan is a highly challenging task. As Sherlock Holmes noted:

Most people, if you describe a train of events to them, will tell you what the result would be. They can put those events together in their minds, and argue from them that something will come to pass. There are few people, however, who, if you told them a result, would be able to evolve from their own inner consciousness what the steps were which led up to that result. This power is what I mean when I talk of reasoning backward. [1]

Like a detective, a planner begins with a result (i.e. a facility design) and must synthesize the steps required to yield this result. Essential aspects of construction planning include the generation of required activities, analysis of the implications of these activities, and choice among the various alternative means of performing activities. In contrast to a detective discovering a single train of events, however, construction planners also face the normative problem of choosing the best among numerous alternative plans. Moreover, a detective is faced with an observable result, whereas a planner must imagine the final facility as described in the plans and specifications.

In developing a construction plan, it is common to adopt a primary emphasis on either cost control or on schedule control as illustrated in Fig. 9-1. Some projects are primarily divided into expense categories with associated costs. In these cases, construction planning is cost or expense oriented. Within the categories of expenditure, a distinction is made between costs incurred directly in the performance of an activity and indirectly for the accomplishment of the project. For example, borrowing expenses for project financing and overhead items are commonly treated as indirect costs. For other projects, scheduling of work activities over time is critical and is emphasized in the planning process. In this case, the planner insures that the proper precedences among activities are maintained and that efficient scheduling of the available resources prevails. Traditional scheduling procedures emphasize the maintenance of task precedences (resulting in critical path scheduling procedures) or efficient use of resources over time (resulting in job shop scheduling procedures). Finally, most complex projects require consideration of both cost and scheduling over time, so that planning, monitoring and record keeping must consider both dimensions. In these cases, the integration of schedule and budget information is a major concern.

**Figure 9-1 Alternative Emphases in Construction Planning**

In this chapter, we shall consider the functional requirements for construction planning such as technology choice, work breakdown, and budgeting.
Construction planning is not an activity which is restricted to the period after the award of a contract for construction. It should be an essential activity during the facility design. Also, if problems arise during construction, re-planning is required.

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9.2 Choice of Technology and Construction Method

As in the development of appropriate alternatives for facility design, choices of appropriate technology and methods for construction are often ill-structured yet critical ingredients in the success of the project. For example, a decision whether to pump or to transport concrete in buckets will directly affect the cost and duration of tasks involved in building construction. A decision between these two alternatives should consider the relative costs, reliabilities, and availability of equipment for the two transport methods. Unfortunately, the exact implications of different methods depend upon numerous considerations for which information may be sketchy during the planning phase, such as the experience and expertise of workers or the particular underground condition at a site.

In selecting among alternative methods and technologies, it may be necessary to formulate a number of construction plans based on alternative methods or assumptions. Once the full plan is available, then the cost, time and reliability impacts of the alternative approaches can be reviewed. This examination of several alternatives is often made explicit in bidding competitions in which several alternative designs may be proposed or value engineering for alternative construction methods may be permitted. In this case, potential constructors may wish to prepare plans for each alternative design using the suggested construction method as well as to prepare plans for alternative construction methods which would be proposed as part of the value engineering process.

In forming a construction plan, a useful approach is to simulate the construction process either in the imagination of the planner or with a formal computer based simulation technique. [2] By observing the result, comparisons among different plans or problems with the existing plan can be identified. For example, a decision to use a particular piece of equipment for an operation immediately leads to the question of whether or not there is sufficient access space for the equipment. Three dimensional geometric models in a computer aided design (CAD) system may be helpful in simulating space requirements for operations and for identifying any interferences. Similarly, problems in resource availability identified during the simulation of the construction process might be effectively forestalled by providing additional resources as part of the construction plan.

Example 9-1: A roadway rehabilitation

An example from a roadway rehabilitation project in Pittsburgh, PA can serve to illustrate the importance of good construction planning and the effect of technology choice. In this project, the decks on overpass bridges as well as the pavement on the highway itself were to be replaced. The initial construction plan was to work outward from each end of the overpass bridges while the highway surface was replaced below the bridges. As a result, access of equipment and concrete trucks to the overpass bridges was a considerable problem. However, the highway work could be staged so that each overpass bridge was accessible from below at prescribed times. By pumping concrete up to the overpass bridge deck from the highway below, costs were reduced and the work was accomplished much more quickly.

Example 9-2: Laser Leveling

An example of technology choice is the use of laser leveling equipment to improve the productivity of excavation and grading. [3] In these systems, laser surveying equipment is erected on a site so that the relative height of mobile equipment is known exactly. This height measurement is accomplished by flashing a rotating laser light on a level plane across the construction site and observing exactly where the light shines on receptors on mobile equipment such as graders. Since laser light does not disperse appreciably, the height at which the laser shines anywhere on the construction site gives an accurate indication of the height of a receptor on a piece of mobile equipment. In turn, the receptor height can be used to measure the height of a blade, excavator bucket or other piece of equipment. Combined with electro-hydraulic control systems mounted on mobile equipment such as bulldozers, graders and scrapers, the height of excavation and grading blades can be precisely and automatically controlled in these systems. This automation of blade heights has reduced costs in some cases by over 80% and improved quality in the finished product, as measured by the desired amount of excavation or the extent to which a final grade achieves the desired angle. These systems also permit the use of smaller machines and less skilled operators. However, the use of these semi-automated systems require investments in the laser surveying equipment as well as modification to equipment to permit electronic feedback control units. Still, laser leveling appears to be an excellent technological choice in many instances.
9.3 Defining Work Tasks

At the same time that the choice of technology and general method are considered, a parallel step in the planning process is to define the various work tasks that must be accomplished. These work tasks represent the necessary framework to permit scheduling of construction activities, along with estimating the resources required by the individual work tasks, and any necessary precedences or required sequence among the tasks. The terms work "tasks" or "activities" are often used interchangeably in construction plans to refer to specific, defined items of work. In job shop or manufacturing terminology, a project would be called a "job" and an activity called an "operation", but the sense of the terms is equivalent. [4] The scheduling problem is to determine an appropriate set of activity start time, resource allocations and completion times that will result in completion of the project in a timely and efficient fashion. Construction planning is the necessary fore-runner to scheduling. In this planning, defining work tasks, technology and construction method is typically done either simultaneously or in a series of iterations.

The definition of appropriate work tasks can be a laborious and tedious process, yet it represents the necessary information for application of formal scheduling procedures. Since construction projects can involve thousands of individual work tasks, this definition phase can also be expensive and time consuming. Fortunately, many tasks may be repeated in different parts of the facility or past facility construction projects. For example, the tasks involved in the construction of a building floor may be repeated with only minor differences for each of the floors in the building. Also, standard definitions and nomenclatures for most tasks exist. As a result, the individual planner defining work tasks does not have to approach each facet of the project entirely from scratch.

While repetition of activities in different locations or reproduction of activities from past projects reduces the work involved, there are very few computer aids for the process of defining activities. Databases and information systems can assist in the storage and recall of the activities associated with past projects as described in Chapter 14. For the scheduling process itself, numerous computer programs are available. But for the important task of defining activities, reliance on the skill, judgment and experience of the construction planner is likely to continue.

More formally, an activity is any subdivision of project tasks. The set of activities defined for a project should be comprehensive or completely exhaustive so that all necessary work tasks are included in one or more activities. Typically, each design element in the planned facility will have one or more associated project activities. Execution of an activity requires time and resources, including manpower and equipment, as described in the next section. The time required to perform an activity is called the duration of the activity. The beginning and the end of activities are signposts or milestones, indicating the progress of the project. Occasionally, it is useful to define activities which have no duration to mark important events. For example, receipt of equipment on the construction site may be defined as an activity since other activities would depend upon the equipment availability and the project manager might appreciate formal notice of the arrival. Similarly, receipt of regulatory approvals would also be specially marked in the project plan.

The extent of work involved in any one activity can vary tremendously in construction project plans. Indeed, it is common to begin with fairly coarse definitions of activities and then to further sub-divide tasks as the plan becomes better defined. As a result, the definition of activities evolves during the preparation of the plan. A result of this process is a natural hierarchy of activities with large, abstract functional activities repeatedly sub-divided into more and more specific sub-tasks. For example, the problem of placing concrete on site would have sub-activities associated with placing forms, installing reinforcing steel, pouring concrete, finishing the concrete, removing forms and others. Even more specifically, sub-tasks such as removal and cleaning of forms after concrete placement can be defined. Even further, the sub-task "clean concrete forms" could be subdivided into the various operations:

- Transport forms from on-site storage and unload onto the cleaning station.
- Position forms on the cleaning station.
- Wash forms with water.
- Clean concrete debris from the form's surface.
- Coat the form surface with an oil release agent for the next use.
- Unload the form from the cleaning station and transport to the storage location.

This detailed task breakdown of the activity "clean concrete forms" would not generally be done in standard construction planning, but it is essential in the process of programming or designing a robot to undertake this activity since the various specific tasks must be well defined for a robot implementation. [5]

It is generally advantageous to introduce an explicit hierarchy of work activities for the purpose of simplifying the presentation and development of a schedule. For example, the initial plan might define a single activity associated with "site clearance." Later, this single activity might be sub-divided into "relocating utilities," "removing vegetation," "grading," etc. However, these activities could continue to be identified as sub-activities under the general activity

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of "site clearance." This hierarchical structure also facilitates the preparation of summary charts and reports in which detailed operations are combined into aggregate or "super"-activities.

More formally, a hierarchical approach to work task definition decomposes the work activity into component parts in the form of a tree. Higher levels in the tree represent decision nodes or summary activities, while branches in the tree lead to smaller components and work activities. A variety of constraints among the various nodes may be defined or imposed, including precedence relationships among different tasks as defined below. Technology choices may be decomposed to decisions made at particular nodes in the tree. For example, choices on plumbing technology might be made without reference to choices for other functional activities.

Of course, numerous different activity hierarchies can be defined for each construction plan. For example, upper level activities might be related to facility components such as foundation elements, and then lower level activity divisions into the required construction operations might be made. Alternatively, upper level divisions might represent general types of activities such as electrical work, while lower work divisions represent the application of these operations to specific facility components. As a third alternative, initial divisions might represent different spatial locations in the planned facility. The choice of a hierarchy depends upon the desired scheme for summarizing work information and on the convenience of the planner. In computerized databases, multiple hierarchies can be stored so that different aggregations or views of the work breakdown structure can be obtained.

The number and detail of the activities in a construction plan is a matter of judgment or convention. Construction plans can easily range between less than a hundred to many thousand defined tasks, depending on the planner's decisions and the scope of the project. If subdivided activities are too refined, the size of the network becomes unwieldy and the cost of planning excessive. Sub-division yields no benefit if reasonably accurate estimates of activity durations and the required resources cannot be made at the detailed work breakdown level. On the other hand, if the specified activities are too coarse, it is impossible to develop realistic schedules and details of resource requirements during the project. More detailed task definitions permit better control and more realistic scheduling. It is useful to define separate work tasks for:

- those activities which involve different resources, or
- those activities which do not require continuous performance.

For example, the activity "prepare and check shop drawings" should be divided into a task for preparation and a task for checking since different individuals are involved in the two tasks and there may be a time lag between preparation and checking.

In practice, the proper level of detail will depend upon the size, importance and difficulty of the project as well as the specific scheduling and accounting procedures which are adopted. However, it is generally the case that most schedules are prepared with too little detail than too much. It is important to keep in mind that task definition will serve as the basis for scheduling, for communicating the construction plan and for construction monitoring. Completion of tasks will also often serve as a basis for progress payments from the owner. Thus, more detailed task definitions can be quite useful. But more detailed task breakdowns are only valuable to the extent that the resources required, durations and activity relationships are realistically estimated for each activity. Providing detailed work task breakdowns is not helpful without a commensurate effort to provide realistic resource requirement estimates. As more powerful, computer-based scheduling and monitoring procedures are introduced, the ease of defining and manipulating tasks will increase, and the number of work tasks can reasonably be expected to expand.

**Example 9-3: Task Definition for a Road Building Project**

As an example of construction planning, suppose that we wish to develop a plan for a road construction project including two culverts. [6] Initially, we divide project activities into three categories as shown in Figure 9-2: structures, roadway, and general. This division is based on the major types of design elements to be constructed. Within the roadway work, a further sub-division is into earthwork and pavement. Within these subdivisions, we identify clearing, excavation, filling and finishing (including seeding and sodding) associated with earthwork, and we define watering, compaction and paving sub-activities associated with pavement. Finally, we note that the roadway segment is fairly long, and so individual activities can be defined for different physical segments along the roadway path. In Figure 9-2, we divide each paving and earthwork activity into activities specific to each of two roadway segments. For the culvert construction, we define the subdivisions of structural excavation, concreting, and reinforcing. Even more specifically, structural excavation is divided into excavation itself and the required backfill and compaction. Similarly, concreting is divided into placing concrete forms, pouring concrete, stripping forms, and curing the concrete. As a final step in the structural planning, detailed activities are defined for reinforcing each of the two culverts. General work activities are defined for move in, general supervision, and clean up. As a
result of this planning, over thirty different detailed activities have been defined.

At the option of the planner, additional activities might also be defined for this project. For example, materials ordering or lane striping might be included as separate activities. It might also be the case that a planner would define a different hierarchy of work breakdowns than that shown in Figure 9-2. For example, placing reinforcing might have been a subactivity under concreting for culverts. One reason for separating reinforcement placement might be to emphasize the different material and resources required for this activity. Also, the division into separate roadway segments and culverts might have been introduced early in the hierarchy. With all these potential differences, the important aspect is to insure that all necessary activities are included somewhere in the final plan.

More complicated precedence relationships can also be specified. For example, one activity might not be able to start for several days after the completion of another activity. As a common example, concrete might have to cure (or set) for several days before formwork is removed. This restriction on the removal of forms activity is called a lag between the completion of one activity (i.e., pouring concrete in this case) and the start of another activity (i.e., removing formwork in this case). Many computer based scheduling programs permit the use of a variety of precedence relationships.

Three mistakes should be avoided in specifying predecessor relationships for construction plans. First, a circle of activity precedences will result in an impossible plan. For example, if activity A precedes activity B, activity B precedes activity C, and activity C precedes activity A, then the project can never be started or completed! Figure 9-4 illustrates the resulting activity network. Fortunately, formal scheduling methods and good computer scheduling programs will find any such errors in the logic of the construction plan.

Forgetting a necessary precedence relationship can be more insidious. For example, suppose that installation of dry wall should be done prior to floor finishing. Ignoring this precedence relationship may result in both activities being scheduled at the same time. Corrections on the spot may result in increased costs or problems of quality in the completed project. Unfortunately, there are few ways in which precedence omissions can be found other than with checks by knowledgeable managers or by comparison.
to comparable projects. One other possible but little used mechanism for checking precedences is to conduct a physical or computer based simulation of the construction process and observe any problems.

Finally, it is important to realize that different types of precedence relationships can be defined and that each has different implications for the schedule of activities:

- Some activities have a necessary technical or physical relationship that cannot be superseded. For example, concrete pours cannot proceed before formwork and reinforcement are in place.
- Some activities have a necessary precedence relationship over a continuous space rather than as discrete work task relationships. For example, formwork may be placed in the first part of an excavation trench even as the excavation equipment continues to work further along in the trench. Formwork placement cannot proceed further than the excavation, but the two activities can be started and stopped independently within this constraint.
- Some "precedence relationships" are not technically necessary but are imposed due to implicit decisions within the construction plan. For example, two activities may require the same piece of equipment so a precedence relationship might be defined between the two to insure that they are not scheduled for the same time period. Which activity is scheduled first is arbitrary. As a second example, reversing the sequence of two activities may be technically possible but more expensive. In this case, the precedence relationship is not physically necessary but only applied to reduce costs as perceived at the time of scheduling.

In revising schedules as work proceeds, it is important to realize that different types of precedence relationships have quite different implications for the flexibility and cost of changing the construction plan. Unfortunately, many formal scheduling systems do not possess the capability of indicating this type of flexibility. As a result, the burden is placed upon the manager of making such decisions and insuring realistic and effective schedules. With all the other responsibilities of a project manager, it is no surprise that preparing or revising the formal, computer based construction plan is a low priority to a manager in such cases. Nevertheless, formal construction plans may be essential for good management of complicated projects.

Example 9-4: Precedence Definition for Site Preparation and Foundation Work

Suppose that a site preparation and concrete slab foundation construction project consists of nine different activities:

- A. Site clearing (of brush and minor debris),
- B. Removal of trees,
- C. General excavation,
- D. Grading general area,
- E. Excavation for utility trenches,
- F. Placing formwork and reinforcement for concrete,
- G. Installing sewer lines,
- H. Installing other utilities,
- I. Pouring concrete.

Activities A (site clearing) and B (tree removal) do not have preceding activities since they depend on none of the other activities. We assume that activities C (general excavation) and D (general grading) are preceded by activity A (site clearing). It might also be the case that the planner wished to delay any excavation until trees were removed, so that B (tree removal) would be a precedent activity to C (general excavation) and D (general grading). Activities E (trench excavation) and F (concrete preparation) cannot begin until the completion of general excavation and tree removal, since they involve subsequent excavation and trench preparation. Activities G (install lines) and H (install utilities) represent installation in the utility trenches and cannot be attempted until the trenches are prepared, so that activity E (trench excavation) is a preceding activity. We also assume that the utilities should not be installed until grading is completed to avoid equipment conflicts, so activity D (general grading) is also preceding activities G (install sewers) and H (install utilities). Finally, activity I (pour concrete) cannot begin until the sewer line is installed and formwork and reinforcement are ready, so activities F and G are preceding. Other utilities may be routed over the slab foundation, so activity H (install utilities) is not necessarily a preceding activity for activity I (pour concrete). The result of our planning are the immediate precedences shown in Table 9-1.

<table>
<thead>
<tr>
<th>Activity</th>
<th>Description</th>
<th>Predecessors</th>
</tr>
</thead>
<tbody>
<tr>
<td>A</td>
<td>Site clearing</td>
<td>---</td>
</tr>
<tr>
<td>B</td>
<td>Removal of trees</td>
<td>---</td>
</tr>
<tr>
<td>C</td>
<td>General excavation</td>
<td>A</td>
</tr>
<tr>
<td>D</td>
<td>Grading general area</td>
<td>A</td>
</tr>
<tr>
<td>E</td>
<td>Excavation for utility trenches</td>
<td>B,C</td>
</tr>
<tr>
<td>F</td>
<td>Placing formwork and reinforcement for concrete</td>
<td>B,C</td>
</tr>
<tr>
<td>G</td>
<td>Installing sewer lines</td>
<td>D,E</td>
</tr>
<tr>
<td>H</td>
<td>Installing other utilities</td>
<td>D,E,F,G</td>
</tr>
<tr>
<td>I</td>
<td>Pouring concrete</td>
<td></td>
</tr>
</tbody>
</table>
With this information, the next problem is to represent the activities in a network diagram and to determine all the precedence relationships among the activities. One network representation of these nine activities is shown in Figure 9-5, in which the activities appear as branches or links between nodes. The nodes represent milestones of possible beginning and starting times. This representation is called an activity-on-branch diagram. Note that an initial event beginning activity is defined (Node 0 in Figure 9-5), while node 5 represents the completion of all activities.

It is also notable that Table 9-1 lists only the immediate predecessor relationships. Clearly, there are other precedence relationships which involve more than one activity. For example, "installing sewer lines" (activity G) cannot be undertaken before "site clearing" (Activity A) is complete since the activity "grading general area" (Activity D) must precede activity G and must follow activity A. Table 9-1 is an implicit precedence list since only immediate predecessors are recorded. An explicit predecessor list would include all of the preceding activities for activity G. Table 9-2 shows all such predecessor relationships implied by the project plan. This table can be produced by tracing all paths through the network back from a particular activity and can be performed algorithmically. For example, inspecting Figure 9-6 reveals that each activity except for activity B depends upon the completion of activity A.

### Table 9-2 All Activity Precedence Relationships for a Nine-Activity Project

<table>
<thead>
<tr>
<th>Predecessor Activity</th>
<th>Direct Successor Activities</th>
<th>All Successor Activities</th>
<th>All Predecessor Activities</th>
</tr>
</thead>
<tbody>
<tr>
<td>A</td>
<td>C,D</td>
<td>E,F,G,H,I</td>
<td>---</td>
</tr>
<tr>
<td>B</td>
<td>E,F</td>
<td>G,H,I</td>
<td>A</td>
</tr>
<tr>
<td>C</td>
<td>E,F</td>
<td>G,H,I</td>
<td>A</td>
</tr>
<tr>
<td>D</td>
<td>G,H</td>
<td>I</td>
<td>A</td>
</tr>
<tr>
<td>E</td>
<td>G,H</td>
<td>I</td>
<td>A,B,C</td>
</tr>
<tr>
<td>F</td>
<td>I</td>
<td>---</td>
<td>A,B,C</td>
</tr>
<tr>
<td>G</td>
<td>I</td>
<td>---</td>
<td>A,B,C,D,E</td>
</tr>
<tr>
<td>H</td>
<td>---</td>
<td>---</td>
<td>A,B,C,D,E</td>
</tr>
<tr>
<td>I</td>
<td>---</td>
<td>---</td>
<td>A,B,C,D,E,F,G</td>
</tr>
</tbody>
</table>

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### 9.5 Estimating Activity Durations

In most scheduling procedures, each work activity has an associated time duration. These durations are used extensively in preparing a schedule. For example, suppose that the durations shown in Table 9-3 were estimated for the project diagrammed in Figure 9-0. The entire set of activities would then require at least 3 days, since the activities follow one another directly and require a total of $1.0 + 0.5 + 0.5 + 1.0 = 3$ days. If another activity proceeded in parallel with this sequence, the 3 day minimum duration of these four activities is unaffected. More than 3 days would be required for the sequence if there was a delay or a lag between the completion of one activity and the start of another.
TABLE 9-3 Durations and Predecessors for a Four Activity Project Illustration

<table>
<thead>
<tr>
<th>Activity</th>
<th>Predecessor</th>
<th>Duration (Days)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Excavate trench</td>
<td>---</td>
<td>1.0</td>
</tr>
<tr>
<td>Place formwork</td>
<td>Excavate trench</td>
<td>0.5</td>
</tr>
<tr>
<td>Place reinforcing</td>
<td>Place formwork</td>
<td>0.5</td>
</tr>
<tr>
<td>Pour concrete</td>
<td>Place reinforcing</td>
<td>1.0</td>
</tr>
</tbody>
</table>

All formal scheduling procedures rely upon estimates of the durations of the various project activities as well as the definitions of the predecessor relationships among tasks. The variability of an activity's duration may also be considered. Formally, the probability distribution of an activity's duration as well as the expected or most likely duration may be used in scheduling. A probability distribution indicates the chance that a particular activity duration will occur. In advance of actually doing a particular task, we cannot be certain exactly how long the task will require.

A straightforward approach to the estimation of activity durations is to keep historical records of particular activities and rely on the average durations from this experience in making new duration estimates. Since the scope of activities are unlikely to be identical between different projects, unit productivity rates are typically employed for this purpose. For example, the duration of an activity $D_{ij}$ such as concrete formwork assembly might be estimated as:

$$D_{ij} = \frac{A_{ij}}{P_{ij}N_{ij}}$$

(9.1)

where $A_{ij}$ is the required formwork area to assemble (in square yards), $P_{ij}$ is the average productivity of a standard crew in this task (measured in square yards per hour), and $N_{ij}$ is the number of crews assigned to the task. In some organizations, unit production time, $T_{ij}$, is defined as the time required to complete a unit of work by a standard crew (measured in hours per square yards) is used as a productivity measure such that $T_{ij}$ is a reciprocal of $P_{ij}$.

A formula such as Eq. (9.1) can be used for nearly all construction activities. Typically, the required quantity of work, $A_{ij}$ is determined from detailed examination of the final facility design. This quantity-take-off to obtain the required amounts of materials, volumes, and areas is a very common process in bid preparation by contractors. In some countries, specialized quantity surveyors provide the information on required quantities for all potential contractors and the owner. The number of crews working, $N_{ij}$, is decided by the planner. In many cases, the number or amount of resources applied to particular activities may be modified in light of the resulting project plan and schedule. Finally, some estimate of the expected work productivity, $P_{ij}$ must be provided to apply Equation (9.1). As with cost factors, commercial services can provide average productivity figures for many standard activities of this sort. Historical records in a firm can also provide data for estimation of productivities.

The calculation of a duration as in Equation (9.1) is only an approximation to the actual activity duration for a number of reasons. First, it is usually the case that peculiarities of the project make the accomplishment of a particular activity more or less difficult. For example, access to the forms in a particular location may be difficult; as a result, the productivity of assembling forms may be lower than the average value for a particular project. Often, adjustments based on engineering judgment are made to the calculated durations from Equation (9.1) for this reason.

In addition, productivity rates may vary in both systematic and random fashions from the average. An example of systematic variation is the effect of learning on productivity. As a crew becomes familiar with an activity and the work habits of the crew, their productivity will typically improve. Figure 9-7 illustrates the type of productivity increase that might occur with experience; this curve is called a learning curve. The result is that productivity $P_{ij}$ is a function of the duration of an activity or project. A common construction example is that the assembly of floors in a building might go faster at higher levels due to improved productivity even though the transportation time up to the active construction area is longer. Again, historical records or subjective adjustments might be made to represent learning curve variations in average productivity. [8]
Random factors will also influence productivity rates and make estimation of activity durations uncertain. For example, a scheduler will typically not know at the time of making the initial schedule how skillful the crew and manager will be that are assigned to a particular project. The productivity of a skilled designer may be many times that of an unskilled engineer. In the absence of specific knowledge, the estimator can only use average values of productivity.

Weather effects are often very important and thus deserve particular attention in estimating durations. Weather has both systematic and random influences on activity durations. Whether or not a rainstorm will come on a particular day is certainly a random effect that will influence the productivity of many activities. However, the likelihood of a rainstorm is likely to vary systematically from one month or one site to the next. Adjustment factors for inclement weather as well as meteorological records can be used to incorporate the effects of weather on durations. As a simple example, an activity might require ten days in perfect weather, but the activity could not proceed in the rain. Furthermore, suppose that rain is expected ten percent of the days in a particular month. In this case, the expected activity duration is eleven days including one expected rain day.

Finally, the use of average productivity factors themselves cause problems in the calculation presented in Equation (9.1). The expected value of the multiplicative reciprocal of a variable is not exactly equal to the reciprocal of the variable's expected value. For example, if productivity on an activity is either six in good weather (ie., $P=6$) or two in bad weather (ie., $P=2$) and good or bad weather is equally likely, then the expected productivity is $P = (6)(0.5) + (2)(0.5) = 4$, and the reciprocal of expected productivity is $1/4$. However, the expected reciprocal of productivity is $E[1/P] = (0.5)/6 + (0.5)/2 = 1/3$. The reciprocal of expected productivity is 25% less than the expected value of the reciprocal in this case! By representing only two possible productivity values, this example represents an extreme case, but it is always true that the use of average productivity factors in Equation (9.1) will result in optimistic estimates of activity durations. The use of actual averages for the reciprocals of productivity or small adjustment factors may be used to correct for this non-linearity problem.

The simple duration calculation shown in Equation (9.1) also assumes an inverse linear relationship between the number of crews assigned to an activity and the total duration of work. While this is a reasonable assumption in situations for which crews can work independently and require no special coordination, it need not always be true. For example, design tasks may be divided among numerous architects and engineers, but delays to insure proper coordination and communication increase as the number of workers increase. As another example, insuring a smooth flow of material to all crews on a site may be increasingly difficult as the number of crews increase. In these latter cases, the relationship between activity duration and the number of crews is unlikely to be inversely proportional as shown in Equation (9.1). As a result, adjustments to the estimated productivity from Equation (9.1) must be made. Alternatively, more complicated functional relationships might be estimated between duration and resources used in the same way that nonlinear preliminary or conceptual cost estimate models are prepared.

One mechanism to formalize the estimation of activity durations is to employ a hierarchical estimation framework. This approach decomposes the estimation problem into component parts in which the higher levels in the hierarchy represent attributes which depend upon the details of lower level adjustments and calculations. For example, Figure 9-8 represents various levels in the estimation of the duration of masonry construction. At the lowest level, the maximum productivity for the activity is estimated based upon general work conditions. Table 9-4 illustrates some possible maximum productivity values that might be employed in this estimation. At the next higher level, adjustments to these maximum productivities are made to account for special site conditions and crew compositions; table 9-5 illustrates some possible adjustment rules. At the highest level,
adjustments for overall effects such as weather are introduced. Also shown in Figure 9-8 are nodes to estimate down or unproductive time associated with the masonry construction activity. The formalization of the estimation process illustrated in Figure 9-8 permits the development of computer aids for the estimation process or can serve as a conceptual framework for a human estimator.

### TABLE 9-4 Maximum Productivity Estimates for Masonry Work

<table>
<thead>
<tr>
<th>Masonry unit size</th>
<th>Condition(s)</th>
<th>Maximum productivity achievable</th>
</tr>
</thead>
<tbody>
<tr>
<td>8 inch</td>
<td>None</td>
<td>400 units/day/mason</td>
</tr>
<tr>
<td>6 inch</td>
<td>Wall is &quot;long&quot;</td>
<td>430 units/day/mason</td>
</tr>
<tr>
<td>6 inch</td>
<td>Wall is not &quot;long&quot;</td>
<td>370 units/day/mason</td>
</tr>
<tr>
<td>12 inch</td>
<td>Labor is nonunion</td>
<td>300 units/day/mason</td>
</tr>
<tr>
<td>4 inch</td>
<td>Wall is &quot;long&quot;</td>
<td>480 units/day/mason</td>
</tr>
<tr>
<td>4 inch</td>
<td>Wall is not &quot;long&quot;</td>
<td>430 units/day/mason</td>
</tr>
<tr>
<td>4 inch</td>
<td>Weather is &quot;warm and dry&quot; or high-strength mortar is used</td>
<td>370 units/day/mason</td>
</tr>
<tr>
<td>8 inch</td>
<td>There is support from existing wall</td>
<td>1,000 units/day/mason</td>
</tr>
<tr>
<td>8 inch</td>
<td>There is no support from existing wall</td>
<td>750 units/day/mason</td>
</tr>
</tbody>
</table>

### TABLE 9-5 Possible Adjustments to Maximum Productivities for Masonry Construction

<table>
<thead>
<tr>
<th>Impact</th>
<th>Condition(s)</th>
<th>Adjustment magnitude (% of maximum)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Crew type</td>
<td>Crew type is nonunion Job is &quot;large&quot;</td>
<td>15%</td>
</tr>
<tr>
<td>Crew type</td>
<td>Crew type is union Job is &quot;small&quot;</td>
<td>10%</td>
</tr>
<tr>
<td>Supporting labor</td>
<td>There are less than two laborers per crew</td>
<td>20%</td>
</tr>
<tr>
<td>Elevation</td>
<td>Steel frame building with masonry exterior wall has &quot;insufficient&quot; support labor</td>
<td>10%</td>
</tr>
<tr>
<td>Solid masonry building with work on exterior uses nonunion labor</td>
<td>12%</td>
<td></td>
</tr>
<tr>
<td>Visibility</td>
<td>block is not covered</td>
<td>7%</td>
</tr>
<tr>
<td>Temperature</td>
<td>Temperature is below 45°F</td>
<td>15%</td>
</tr>
<tr>
<td>Temperature</td>
<td>Temperature is above 45°F</td>
<td>10%</td>
</tr>
<tr>
<td>Brick texture</td>
<td>bricks are baked high Weather is cold or moist</td>
<td>10%</td>
</tr>
</tbody>
</table>

Figure 9-8 A Hierarchical Estimation Framework for Masonry Construction
In addition to the problem of estimating the expected duration of an activity, some scheduling procedures explicitly consider the uncertainty in activity duration estimates by using the probabilistic distribution of activity durations. That is, the duration of a particular activity is assumed to be a random variable that is distributed in a particular fashion. For example, an activity duration might be assumed to be distributed as a normal or a beta distributed random variable as illustrated in Figure 9-9. This figure shows the probability or chance of experiencing a particular activity duration based on a probabilistic distribution. The beta distribution is often used to characterize activity durations, since it can have an absolute minimum and an absolute maximum of possible duration times. The normal distribution is a good approximation to the beta distribution in the center of the distribution and is easy to work with, so it is often used as an approximation.

If a standard random variable is used to characterize the distribution of activity durations, then only a few parameters are required to calculate the probability of any particular duration. Still, the estimation problem is increased considerably since more than one parameter is required to characterize most of the probabilistic distribution used to represent activity durations. For the beta distribution, three or four parameters are required depending on its generality, whereas the normal distribution requires two parameters.

As an example, the normal distribution is characterized by two parameters, \( \mu \) and \( \sigma \) representing the average duration and the standard deviation of the duration, respectively. Alternatively, the variance of the distribution \( \sigma^2 \) could be used to describe or characterize the variability of duration times; the variance is the value of the standard deviation multiplied by itself. From historical data, these two parameters can be estimated as:

\[
\mu \approx \bar{x} = \frac{\sum_{k=1}^{n} x_k}{n} \tag{9.2}
\]

\[
\sigma^2 \approx \sum_{k=1}^{n} \frac{(x_k - \bar{x})^2}{n-1} \tag{9.3}
\]

where we assume that \( n \) different observations \( x_k \) of the random variable \( x \) are available. This estimation process might be applied to activity durations directly (so that \( x_k \) would be a record of an activity duration \( D_{ij} \) on a past project) or to the estimation of the distribution of productivities (so that \( x_k \) would be a record of the productivity in an activity \( P_i \) on a past project) which, in turn, is used to estimate durations using Equation (9.4). If more accuracy is desired, the estimation equations for mean and standard deviation, Equations (9.2) and (9.3) would be used to estimate the mean and standard deviation of the reciprocal of productivity to avoid non-linear effects. Using estimates of productivities, the standard deviation of activity duration would be calculated as:

\[
\sigma_{ij} \approx \frac{\sigma_{ijP}}{N_{ijP}} \tag{9.4}
\]

where \( \sigma \) is the estimated standard deviation of the reciprocal of productivity that is calculated from Equation (9.3) by substituting \( 1/P \) for \( x \).

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9.6 Estimating Resource Requirements for Work Activities

In addition to precedence relationships and time durations, resource requirements are usually estimated for each activity. Since the work activities
defined for a project are comprehensive, the total resources required for the project are the sum of the resources required for the various activities. By making resource requirement estimates for each activity, the requirements for particular resources during the course of the project can be identified. Potential bottlenecks can thus be identified, and schedule, resource allocation or technology changes made to avoid problems.

Many formal scheduling procedures can incorporate constraints imposed by the availability of particular resources. For example, the unavailability of a specific piece of equipment or crew may prohibit activities from being undertaken at a particular time. Another type of resource is space. A planner typically will schedule only one activity in the same location at the same time. While activities requiring the same space may have no necessary technical precedence, simultaneous work might not be possible. Computational procedures for these various scheduling problems will be described in Chapters 10 and 11. In this section, we shall discuss the estimation of required resources.

The initial problem in estimating resource requirements is to decide the extent and number of resources that might be defined. At a very aggregate level, resources categories might be limited to the amount of labor (measured in man-hours or in dollars), the amount of materials required for an activity, and the total cost of the activity. At this aggregate level, the resource estimates may be useful for purposes of project monitoring and cash flow planning. For example, actual expenditures on an activity can be compared with the estimated required resources to reveal any problems that are being encountered during the course of a project. Monitoring procedures of this sort are described in Chapter 12. However, this aggregate definition of resource use would not reveal bottlenecks associated with particular types of equipment or workers.

More detailed definitions of required resources would include the number and type of both workers and equipment required by an activity as well as the amount and types of materials. Standard resource requirements for particular activities can be recorded and adjusted for the special conditions of particular projects. As a result, the resources types required for particular activities may already be defined. Reliance on historical or standard activity definitions of this type requires a standard coding system for activities.

In making adjustments for the resources required by a particular activity, most of the problems encountered in forming duration estimations described in the previous section are also present. In particular, resources such as labor requirements will vary in proportion to the work productivity, \( P_{ij} \), used to estimate activity durations in Equation (9.1).

Mathematically, a typical estimating equation would be:

\[
R^k_{ij} = D_{ij} N_{ij} U^k_{ij}
\]

where \( R^k_{ij} \) are the resources of type \( k \) required by activity \( ij \), \( D_{ij} \) is the duration of activity \( ij \), \( N_{ij} \) is the number of standard crews allocated to activity \( ij \), and \( U^k_{ij} \) is the amount of resource type \( k \) used per standard crew. For example, if an activity required eight hours with two crews assigned and each crew required three workers, the effort would be \( R = 8 \times 2 \times 3 = 48 \) labor-hours.

From the planning perspective, the important decisions in estimating resource requirements are to determine the type of technology and equipment to employ and the number of crews to allocate to each task. Clearly, assigning additional crews might result in faster completion of a particular activity. However, additional crews might result in congestion and coordination problems, so that work productivity might decline. Further, completing a particular activity earlier might not result in earlier completion of the entire project, as discussed in Chapter 10.